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INTRODUCTION

Edgeloe Engineering has been engaged by Ocean Gardens Pty Ltd to provide Civil Engineering consultancy services to support the preparation of the Development application for the development of Lots 12, 28, &165 Capel Drive Capel for a mixed-use Development.

This report assesses and reports on the servicing requirements and proposed servicing strategy for the proposed development.

The proposed development is shown in Figure 1 and consists of:

- Retention of existing Capel Tavern
- Short Stay accommodation
- Short Stay facilities
- Over 55 accommodation
- Over 55 facilities
- Mixed Use development
- Foreshore easement



Figure 1 Proposed Development



The existing lots 12, 28, &165 will be amalgamated and 3 new green title lots created as shown in Figure 2 and consisting:

- Lot 28 -R60
- Lot 165 R40 & R60
- Lot 12- R60
- Foreshore easement

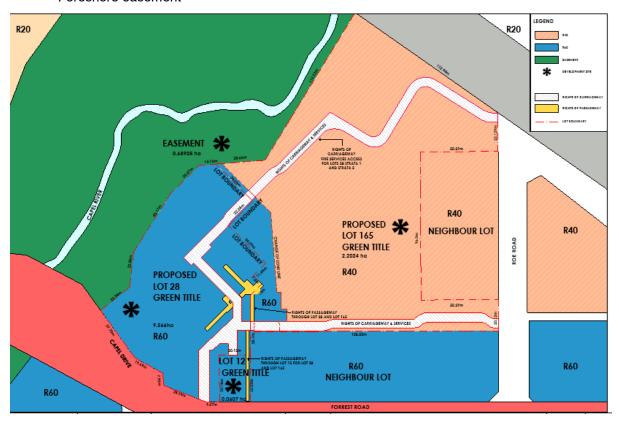


Figure 2 Proposed Lots



2 SITE CONDITIONS

2.1 Existing levels

A full feature survey has been undertaken across the site and a summary of general contours is shown in Figure 3 with the terrain shaded in Figure 4.

The site generally falls from RL 13 at the south east to RL 12.0 along the edge of the river bank and then drops to approximately RL 3.0 along the edge of the Capel River.

There is a depression in the centre of the site at approximately RL 11.8 that appears to be a trapped low area and will require filling. This is evident in Figure 4.

The vegetated central portion of the site is at RL 12.5 and above and these levels would be retained to retain vegetation.



Figure 3 Existing Site Levels



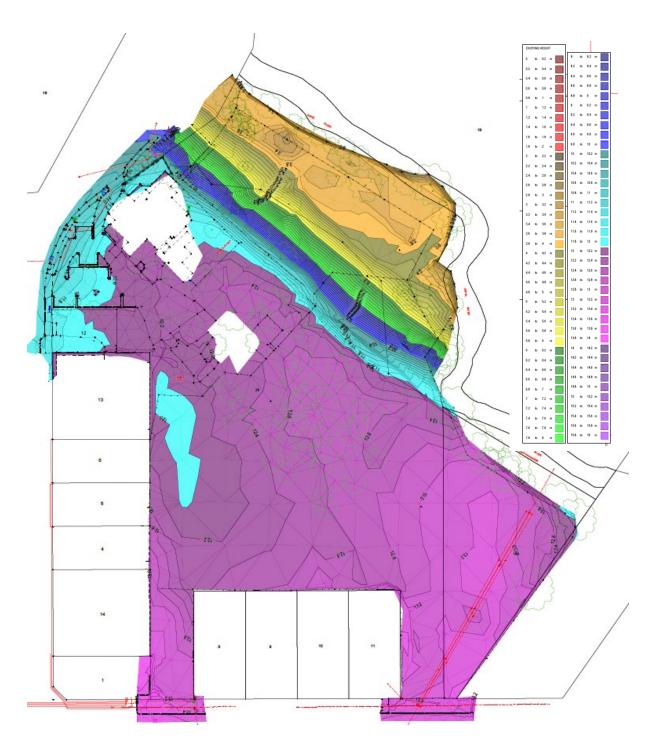


Figure 4 Existing Site Levels

Adjacent the river bank there is a very steep batter section as shown in Figure 5 and this is being further investigated by GALT Geotechnical Engineers to determine slope stability.

The effects of possible continual downstream erosion on the adjacent downstream batter will also be investigated by GALT in the detail design phase and prior to issue of any building approvals for works on the site.



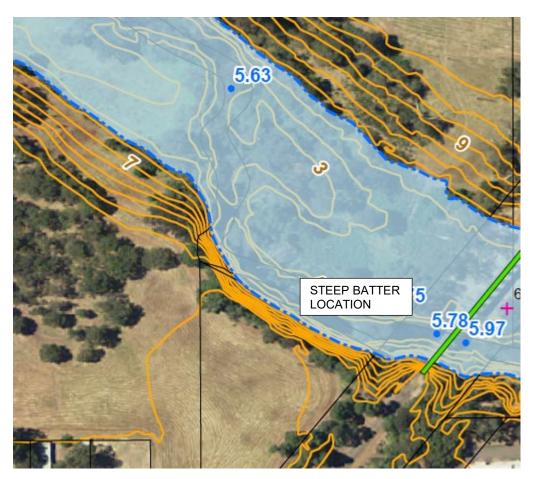


Figure 5 Existing Steep Batter (DWER Flood mapping extract)



2.2 Site Conditions

2.2.1 Geotechnical Investigations

WML (2015) undertook a Geotechnical Investigation across the site and described the site as being located on sandy silts of the Guilford formation, with Bassendean sands underlying the southern limit of the site and alluvial sandy silt at the northern limit of the site in the region of the Capel River.

Based on test pits and soil testing across the site 4 distinct Zones, A1, A2, B & C were determined across the site as shown in Figure 6.



Figure 6 Mapped Soil Zones (WML (2015))

The characteristics of each of the 4 areas was summarised as:

Area A1

Forms the flat paddock portion at the west side of the site. Testing of the materials in this area show the clay to be of medium to high plasticity.

The anticipated seasonal movement in area A1 is expected to be around 60mm and an 'H2' classification is appropriate for the buildings in this portion of the site.



| Depth (m) | Description Area A1 (TP 1, 2, 3, 4, 5, 14) |
|-------------------|---|
| 0.0 - 0.1/0.2 | Dark grey fine to medium grained silty sand top soil with roots |
| 0.1/0.2- 1.7/2.2+ | Dry, variable, grey, brown, orange, hard, sandy silty clays of the Guilford Clay Formation. |
| 1.7/2.2+ - 3.8 | Moist, dense yellow-orange, fine grained silty sand |
| Comments | Groundwater was not encountered in any of the test pits. The lot on Roe Road containing test pit 3 has approximately 0.4-0.5m of uncontrolled sandy clay fill above natural ground. |

Area A2

Forms the flat paddock portion at the east side of the site. Testing of the materials show the clay to be of medium plasticity.

The anticipated seasonal movement in area A2 is expected to be around 30mm and an 'M' classification is appropriate for the buildings in this portion of the site.

| Depth (m) | Description Area A2 (TP 10, 11,12) |
|----------------|---|
| 0 – 0.1 | Dark grey fine to medium grained silty sand top soil with roots |
| 0.1 – 1.8/2.4 | Moist dark red, very stiff sandy clay |
| 1.8/2.4 - 2.8+ | Moist, dense yellow brown, sandy clay |
| Comments | Groundwater was not encountered in any of the test pits. |

Area B

Forms the portion of the site that slopes towards the Capel River. Test pit 8 located on the slope and test pit 13 located at the crest of the slope proved silty sand below topsoil to termination of the hole. The sand encountered in test pit 6 was shallower and underlain by clay from Area A, this potentially identifies a sand filled channel in the vicinity of test pit 6.

The anticipated seasonal movement in area B is expected to be around 15mm and an 'S' classification is appropriate for the buildings in this portion of the site.

| Depth (m) | Description Area B (TP 6, 7, 8, 13) |
|---|--|
| 0.0- 0.1/0.4 | Dark grey silty sand top soil with roots. |
| 0.1/0.4 – 1.6/3.0 | Dry, fine to medium grained sand and silty clayey sand, with some grey mottled brown silty clay lenses Trial pit 8 and 13 was terminated in this sand, |
| 1.75/2.4 + | Moist, dense, yellow-orange, fine grained silty Sand |
| Comments Groundwater was not encountered in any of the test pits. | |

Area C

Forms the flat Portion of the site at the toe of the slope beside the Capel River

The anticipated seasonal movement in area C is expected to be around 30mm. Testing shows the soils to 2.6m contain 7% organic content by weight, and are classified as an organic soil. This area of the site in its current condition is considered unsuitable for development.



| Depth (m) | Description Area C (TP 9) |
|-----------|--|
| 0.0- 0.3 | Dark grey silty sand top soil with roots. |
| 0.3 – 2.9 | Dark brown-dark grey organic clayey fine sand |
| 2.9-3 | Blue green sandy clay with silt |
| Comments | Groundwater was not encountered in any of the test pits. |

It was also advised that Areas A1, A2 can be improved to an "S" classification by placing 800mm of sand fill below the proposed foundation, provided the site preparation is carried out as described below. Area C could be improved to an "S" classification by over excavating 1m of the insitu material below the proposed foundation and a minimum of 1m latterly from the outer edge of the planned foundation and backfilling with compacted clean sand.

An alternative to over excavation would be to place a 1m sand pad above natural ground, however settlement monitoring stations should be installed and construction of footings only commenced once the majority of settlement has occurred.

It is noted that the foundations planned for this development will use alternate methods such as screw piles and so these reported Site Classifications may not be relevant.

WML (2015) also reported the steep slope approximately 20m high formed in soil like materials running parallel with the river that bounded the development site at the north edge of lot 165. They reported the slope exhibits signs of considerable instability, evidence of previous slope failures and current ongoing failures are apparent. They reported currently the slope poses an unacceptable geotechnical risk to a potential development in the vicinity of the slope crest and recommended that further investigation and slope stability analysis is necessary if the land in the vicinity of the slope crest is to be developed.

The proponent for this development has engaged GALT Geotechnical to undertke this investigation and report on any measures that may be required to remove associated risk.

2.2.2 Acid Sulfate Soils

Acid Sulfate Soils were assessed by WML (2015). The DWER ASS risk map as shown in Figure 7 indicates the majority of the site is a Moderate to low risk, however the flat low lying portion of the site next to the Capel River (Area C) is of High to Moderate. Preliminary field testing for ASS conditions undertaken in test pit TP9 and TP8 showed no evidence of ASS. Further investigations would be undertken on this site prior to any ground disturbing works occurring so appropriate ASS management practises can be used.



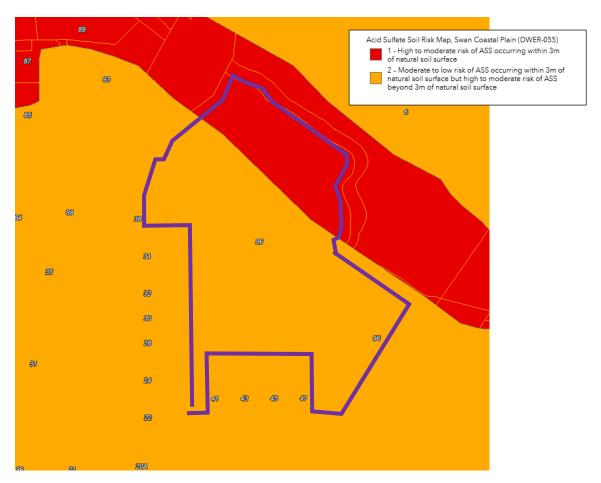


Figure 7 ASS Mapping

2.2.3 Contaminated Sites

The DWER Contaminated sites data base shows a contaminated site to the west of the site as shown in Figure 8.

This site is related to leakage reported from the Service Station on the west side of Capel Drive and Visual and olfactory evidence of hydrocarbons (such as from petrol or diesel) have been identified down-gradient of a service station in the Capel River and along the riverbank.

The report indicates that DWER understands that an underground storage tank (UST) at the service station site failed an integrity test, and it is estimated that approximately 3,450 L of petroleum hydrocarbons leaked from the UST. The location of the UST is yet to be determined. However, information supplied to DWER indicates that the UST is likely to be located in the southern portion of Lot 16 and the south-western portion of the adjacent road reserve.

It is reported that interim management measures were undertaken to reduce off-site impacts to the Capel River, including installation of organic mulch barrier over the riverbank, restriction of access to soil under the bridge using fencing and spill containment within the river in the form of booms and absorbent pads. It is also reported that fencing and spill containment remain in place and that stained soils under the bridge are removed, and absorbent pads are replenished, on a regular basis.

As this is downgradient of the proposed development it is not anticipated to have any impacts on the development.





Figure 8 Site Contamination, DWER Contaminated sites data base.

2.2.3 Groundwater

The WML (2015) investigations did not encounter groundwater but the subsequent Pippin (2019) report indicates that in 2014 and 2015 MPM Developments Consultants undertook two winter periods of groundwater monitoring across the redevelopment site. The monitoring involved the installation of 8 shallow groundwater bores to a depth of 4.0m. The bores were monitored monthly from July to December in 2014 and 2015.

The bore monitoring produced a range of results varying from the highest recorded groundwater levels being found in the South West corner of the property and no record of groundwater being located to a depth of 4.0m along the eastern boundary, adjoining the river foreshore. The highest groundwater peaked at only 400mm below the natural surface in the south western corner in late August 2015.

Pippin (2019) reported that based upon the recorded groundwater levels adjoining this area and nearby surface heights, it is likely that the south western corner contains a perched groundwater and that the heavier clay materials are likely to be trapping rain events with limited permeability through to the deeper groundwater level, that is controlled by Capel River water level.

As part of this development there will be a need to control groundwater levels under roads, paths and buildings and so subsoil drainage in conjunction with fill in select areas will be required.

The extent of subsoil and fill can be refined when a further detailed geotechnical investigation and groundwater monitoring is completed over the site.



2.2.5 Flood mapping

DWER Flood mapping for the Capel River is shown in Figure 9. All development on the site is required to be a minimum of 500mm above the flood levels.

DWER have further recommended that any proposed development is set back from the top of riverbanks to avoid the potential risk of bank erosion during major river flows.

The Proponent has engaged GALT Geotechnical to investigate bank erosion is an issue to be addressed at detail design stage.

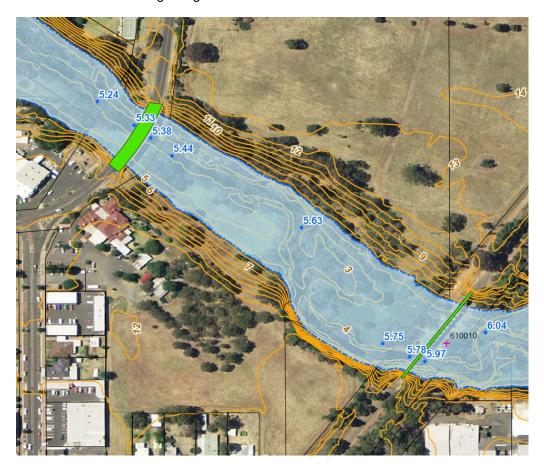


Figure 9 DWER Flood mapping



3 Earthworks

To develop this site fill will be required to achieve:

- Adequate separation above perched groundwater levels
- Grading of the site so the 1% AEP (1 in 100 year) overland flows will be directed in safe flow. paths towards the Capel River.
- Filling to remove trapped low points on the site.
- Suitable subgrade layers below internal roads and paths.
- Building foundation fill
- Landscape area fill

The indicative extent of fill is shown in Figure 10 and will remove the trapped low point within the central portion of the site.



Figure 10 Indicative fill area

This will be further refined at detail design stage and will be determined in conjunction with subsoil drainage on the site to control perched groundwater that could occur on this site.

This fill will require a retaining wall along the southern boundary of the site abutting lots fronting Forrest Road. A Twinside wall of less than 1m high is anticipated. This type of wall is easily removed if required to access the Water Corporation sewer along this alignment for longer term maintenance.



4 SEWER

The site and its existing lots are serviced by Water Corporation sewer network as shown in Figure 11.

Each new lot in the development will be serviced by a single sewer junction located to suit the plumbing depth requirements on the site.

Water Corporation advice has also been sought to ensure there is sufficient capacity in the sewer network to cater for the flows from the proposed development. The response is awaited.

Where proposed works are in proximity to the existing sewers within the lot then buildings and improvements would be undertaken in accordance with the Water Corporation Protection of Assets Technical Guidelines.



Figure 11 Water Corporation Sewer, ESINET (2025)



5 STORMWATER

The stormwater strategy for the area will be designed in accordance with **DWER (2004-2022)** Stormwater Management Manual and the **DWER (2017)** Decision Process for Stormwater Management in Western Australia (2017).

The system will be designed to collect and treat the 1 year 1-hour event in swales or infiltration devices, have the 20% AEP (1 in 5 year) flows captured and detained/ infiltrated on the site and the 1% AEP (1 in 100 year) flows would be safely conveyed to the adjacent Capel River with no additional compensation.

Underground storage devices such as EcoAID and Flo Vault Cells would be used in open space areas and under carparks and walkways to minimise disturbance to existing trees to be retained on site. Flows from these would infiltrate into sand backfill placed around the devices and a trickle outflow from an underlying subsoil system.

Subsoil drains would also be used across the site to control perched groundwater and flows for these would discharge to the storage devices as well.

Runoff from the site would be captured internally in a system of shallow swales, drainage pits and pipes for conveyance to the below ground storage devices.

The indicative drainage scheme is shown in Figure 12 and would be further detailed at the detail design and Building Permit phase of the project.

All drainage would be under private ownership and maintenance and no flows would discharge into Shire of Capel Drainage system adjoining the site without appropriate compensation to acceptable flow rates.



Figure 12 Site Drainage Concept



6 ROADWORKS

All roads within the proposed development will be privately owned and maintained.

Roads, parking and paths within the development would be a mixture of brick paving and asphalt surfacing to tie in with the desired development themes for the site.

Key access points to the site are as shown in Figure 13 and are subject of a separate traffic study.

These include a single access point onto Forrest Road and two access points onto Rowe Road.

These access points also show swept path analysis that has been undertaken by others.

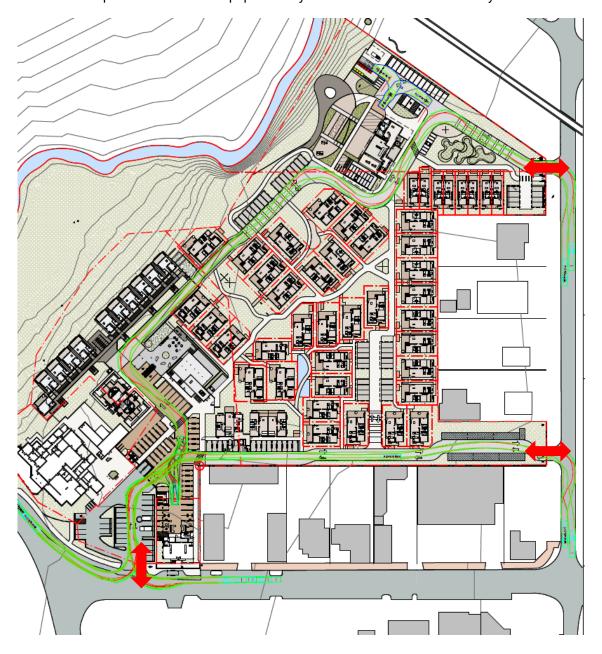


Figure 13 Proposed Access Points



7 WATER SUPPLY

The site and its existing lots are serviced by Water Corporation water network as shown in Figure 14.

There is a 200 RC main on Forrest Road and 200 AC and 150 RC mains along Roe Road.

Each new lot in the development will be serviced by a single water service located to suit the plumbing requirements on the site.

The mains are unlikely to achieve fire pressure and flow requirements and so independent fire storage and boosters are likely required on site for the developments needs.

Water Corporation advice has also been sought to ensure there is sufficient capacity in the water network to cater for the flows from the proposed development. The response is awaited.

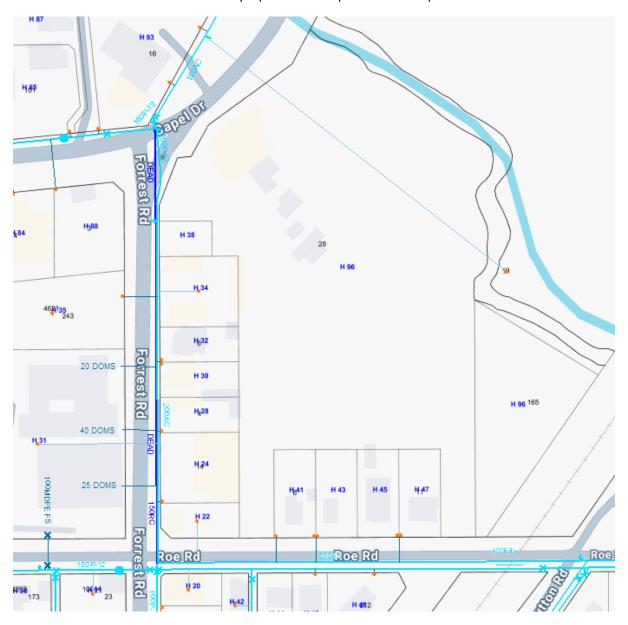


Figure 14 Water Corporation Water, ESINET (2025



8 POWER

The site currently serviced by Western Power overhead and underground network as shown in Figures 15 and 16.

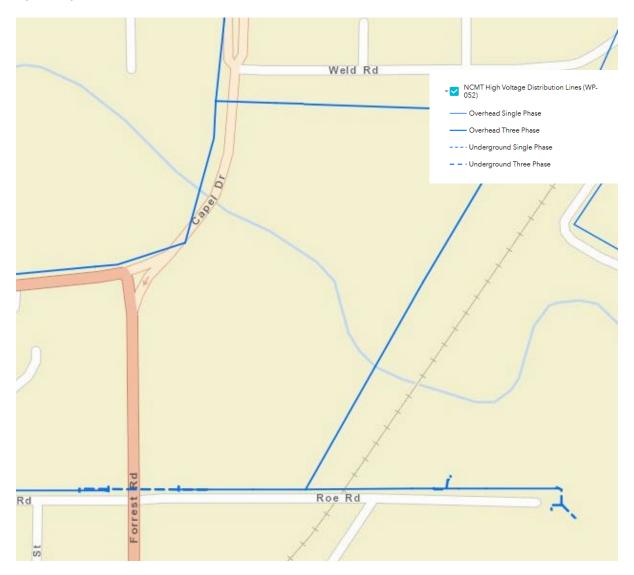


Figure 15 Existing Overhead Power Infrastructure

The power connection to the site will be subject to a formal DIP submission and this will determine any upgrades that may be required in addition to onsite supply requirements such as switchgear and transformers that may be required.

A 3 phase HV overhead line crosses the south eastern portion of the development. A set back will be required from this line so no development occurs within its easement.

A sag study will also be required to ensure sufficient clearance under the power line for all proposed development and road and access crossings.

Low voltage underground cables are also located adjacent the site as shown In Figure 16.



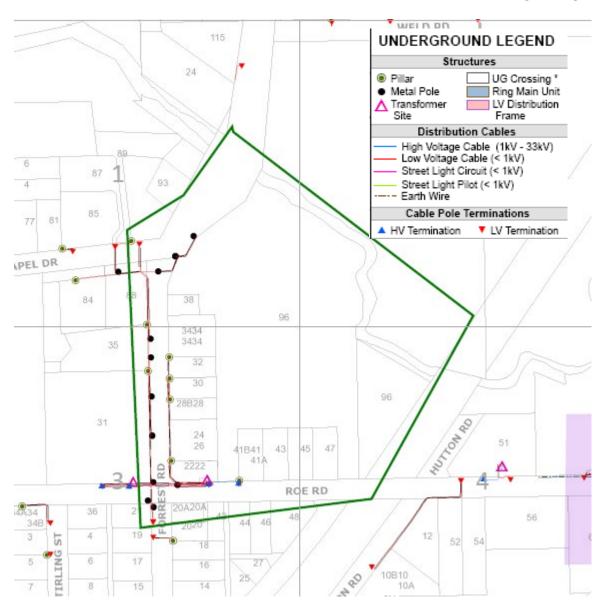


Figure 16 Existing Underground Power Infrastructure

The Western Power Capacity Mapping system shows 15 to 20 MVA available in the system that should be adequate for this development.

9 GAS

Gas is provided to lots in the southern portion of the Capel townsite but here is no gas available in the area of this development.

There is a Gas transmission pipeline located along the rail corridor crossing Roe Road as shown in Figure 17 but this is not suitable for delivery of gas to development without a pressure reduction station being installed.

This pipeline is only 5m off the boundary of Lot 165 and no works are permitted within 15m of the pipeline without prior approval from ATCO.



As works will be undertaken to the boundary of Lot 165 and will fall within this zone ATCO approval will be required.

Gas is not planned to be connected to the site.



Figure 17 Existing Gas Transmission Pipeline

10 NBN

Dial Before Dig records indicate Optus, Telstra and nbn conduits and pits are directly adjacent the site and so a connection should be readily available to the site.



REFERENCES

DWER (2004-2022) Stormwater Management Manual

DWER (2017) Decision Process for Stormwater Management in Western Australia

Pippin Civil Engineering (2019) Engineering Servicing Review and Preliminary Order of magnitude Cost Estimate Capel Tavern Redevelopment

WML (2015) Geotechnical Investigation Lot 12, 28 and 165 Roe Road Capel



Appendix A SITE FEATURE SURVEY

